# PLUMBING DESIGN OF A BUILDING 

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DEPARTMENT OF CIVIL ENGINEERING DAFFODIL INTERNATIONAL UNIVERSITY

# PLUMBING DESIGN OF A BUILDING 

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A Project submitted to the Department of Civil Engineering, Daffodil International University in Partial Fulfillment of the Requirements for the Degree of Bachelor of Science in Civil Engineering


# Department of Civil Engineering <br> Daffodil International University 

October 2022

## APPROVAL

This is to certify that this project "Plumbing design of a building" is done by the following students under my direct supervision and this work has been carried out by them in the Department of Civil Engineering under the Faculty of Engineering of Daffodil International University in partial fulfillment of the requirements for the degree of Bachelor of Science in Civil Engineering.

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## DECLARATION

It is hereby declared that except for the contents where specific reference have been made to the work of others, the design contained in this project report is the result of an detailed design exercise carried out by the author under the supervision of Dr. Miah M. Hussainuzzaman, Associate Professor, Department of Civil Engineering, Daffodil International University.
No part of this project has been submitted to any other university or other educational establishments for a degree, diploma, or other qualification (except for publication).

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Finally, we would like to express a very special indebtedness to our Father and Mother, whose encouragement and support was a continuous source of inspiration for this work.

## DEDICATION

## Dedicated

To

## Our Families


#### Abstract

Plumbing and sanitary system is very important for a building. Proper planning and designing of plumbing system is crucial as it takes care of the hygiene requirements of the occupants. Plumbing isn't only for the collection, transportation, and dispersal of clean water, it can also be used to remove liquids and waste. Plumbers work to ensure that plumbing systems perform waste disposal in an efficient and safe manner. This makes it possible to separate your clean water from harmful contaminants. In this report the plumbing system design for a four store residential building has been presented. This includes the necessary CAD files as well as all the calculations and considerations in detail.


## সারমর্ম

একটি ভবনের জন্য প্লাম্বিং এবং স্যানিটারি সিস্টেম খুবই গুরুত্বপূর্ণ। প্লাম্বিং সিস্টেমের সঠিক পরিকল্পনা এবং নকশা অত্যন্ত গুরুত্বপূর্ণ কারণ এটি বাসিন্দাদের স্বাস্থ্যবিধি প্রয়োজনীয়তার যত্ন নেয়৷ প্লাম্বিং শুধুমাত্র পরিষ্কার জল সংগ্রহ, পরিবহন এবং ছড়িয়ে দেওয়ার জন্য নয়, এটি তরল এবং বর্জ্য অপসারণ করতেও ব্যবহার করা যেতে পারে। প্লাম্বাররা নিশ্চিত করতে কাজ করে যে প্লাম্বিং সিস্টেমগুলি একটি দক্ষ এবং নিরাপদ পদ্ধতিতে বর্জ্য নিষ্পত্তি করে। এটি ক্ষতিকারক দূষক থেকে আপনার পরিষ্কার জল আলাদা করা সম্ভব করে তোলে। এই প্রতিবেদনে চার তলা আবাসিক ভবনের প্লাম্বিং সিস্টেমের নকশা উপস্থাপন করা হয়েছে। এতে প্রয়োজনীয় ক্যাড ফাইলের পাশাপাশি সমস্ত গণনা এবং বিশদ বিবেচনা অন্তর্ভুক্ত রয়েছে।

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## CHAPTER 1 <br> INTRODUCTION

### 1.1 General

All the places where people live needs basic services like water supply and sanitation. In a residential or commercial building where people need to stay for longer period of time requires those services integrated into the building. Proper design and construction of those services ensures economy and safety of the users. Therefore, plumbing design is an integrated part of the detailed design of the building along with architectural, structural electrical designs. Building in large cities may require to have an approved design from the water supply authority to get a water and sewage connection. This project report presents such a design for a residential building.

### 1.2 Objectives

Objective of this report is to present the design of the plumbing system which includes -

- Provide all calculations to determine the right size of different components of this system.
- Provide all the design drawings (CAD) for this system.


### 1.3 Limitations

Some of the features of a complete design is not presented in this report which are:

- Some of the sizing options are fixed by architectural design and this report only verifies those size provided.
- Detailed design of structural elements to hold the plumbing in place is not included.
- Cost and volume estimation is not included.


### 1.4 Organization Of The Report

This report has five chapters, a bibliographic section and appendices. The first chapter describes the general background information, rational, objectives, limitations and
structure of this report. The second chapter provides general information of the specific project. The third, fourth and fifth chapter includes all the calculations and required for water supply, wastewater plumbing and storm water drainage system. Bibliographic section includes the listing of all the references used for these calculations. The the drawings (CAD: Computer Aided Drawing) are provided in the appendices and are referred from the corresponding chapters.

### 1.5 Summary

This chapter of the report introduces the project and the report. The description summarizes all the design that are presented throughout the report along with the objectives and limitations. The following chapter provides the specific project data.

## CHAPTER 2

## GENERAL INFORMATION OF THE PROJECT

### 2.1 GENERAL DATA

| Occupancy type | $:$ Residential |
| :--- | :--- |
| Number of floors | $: 4$ Storied |
| Number of basements | $:$ N/A |
| Building height | $: 14.63$ meters |
| Number of residents | $: 60$ |
| Land Area | $: 1741.10 \mathrm{sft}$ |
| Plinth Area | $: 1148.72 \mathrm{sft}$ |
| Floor Area | $: 443.93 \mathrm{sft}$ |
| Type of structure | $:$ RCC frame |

### 2.2 Location Of The Project

| Address of the project | : Dighinala Sadar, Dighinala-4420 |
| :--- | :--- |
| GPS location | $: 23^{\circ} 15.5^{\prime \prime} \mathrm{N} 92^{\circ} 3.5^{\prime} \mathrm{E}$ |

### 2.3 Plumbing Design Considerations

- BNBC 2020 has been followed for the design.
- Every Flat has balconies.
- Water supply:
- WASA is supplying water to the underground reservoir tank.
- Water distribution system is gravity driven system (not a pressure system)
- Own pumping system will be used to pump the water to the over head tank above the roof.
- Only cold water (no hot water supply) distribution system is considered; anyone can install geyser if they want.
- Water will be pumped to the over head tank (OHT) twice a day.
- All pipe material will be HDPE.
- Sewerage system:
- No sewers are available in the area. Buildings must have their own treatment facility - Septic tank
- Two pipe system is used and hence Soil and Waste water are drained separately.
- Only black water (from soil stack) will be diverted into the septic tank.
- Effluent from septic tank will flow to the open drain along with the wastewater.
- Storm / Rain Water
- There is no rain water harvesting system.
- Rain water from roof and balcony will be collected by separate stack system.
- Rain water will flow to the building drains for wastewater and ultimately will be released to the open drain.


## CHAPTER 3 DESIGN OF WATER SUPPLY SYSTEM

### 3.1 General

A water supply system design requires the following components to be considered:

- Calculation of demand and estimate the water volume.
- Determine / Check adequacy of the underground water reservoir.
- Calculate / select the capacity of the pump to deliver water to the over head tank within a certain time limit (say, about 30 minutes pumping). This will include the pump riser size.
- Determine / Check adequacy of the underground water reservoir.
- Design the distribution pipe line over the roof and the risers.
- Design the distribution pipes over each of the facilities. Generally these pipes run over the false slab of the toilets and through the walls.
- Check the pressure of the most vulnerable water fixture (predicted to have the lowest pressure). This fixture is generally located on the top floor and having the longest supply pipe network.


### 3.1.1 Demand Calculation

Use total people number and per capital demand for specific occupancy type.
Total demand $=$ Population * per capital demand

$$
\Rightarrow \mathrm{Q}=\mathrm{p}^{*} \mathrm{q}
$$

Here, $\quad \mathrm{P}=$ Number of flats/Apartment* 6 persons/flat
$\mathrm{q}=180 \operatorname{Ipcd}$ (for apartment BNBC 2020)
Determined demand for a 4 -store apartment building with 10 flats with 6 persons in each flat:

$$
\begin{aligned}
\mathrm{Q} & =\mathrm{P} * \mathrm{q} \\
& =(5 * 6) * 180 \\
& =5400 \text { Liters } / \text { Day } \\
& =5.4 \mathrm{~m}^{3} / \text { day }
\end{aligned}
$$



Figure 3.1: Project location on the map

### 3.1.2 Under Ground (UG) Reservoir

Generally the Architect provides the design and location for such reservoirs. In such case it is necessary to check the adequacy of the volume provided. A ratio of Volume/Demand provides the security or extra water that can be stored in case of a water supply disruption. The actual volume of the reservoir should include dead volumes at bottom (never be pumped out) and free board (at top, which will never be filled).
Water demand $\quad V=5.4 \mathrm{~m}^{3} /$ day
Require Volume of UG water tank,

$$
\begin{aligned}
\mathrm{V} & =(5.4+5.4) \quad \text { [From BNBC } 2020 \text { Table 5.9.1.2.c] } \\
& =10.8 \mathrm{~m}^{3}
\end{aligned}
$$

Depth of UG water tank,

$$
\mathrm{H}=\mathrm{V}^{13}=(10.8)^{1 / 3}=2.21 \mathrm{~m}
$$

Free board, $\mathrm{FB}=0.15 \mathrm{~m}$
Total depth, $\mathrm{H}=\mathrm{H}+\mathrm{FB}$

$$
=2.21+0.15
$$

$$
=2.36 \mathrm{~m}
$$

Tank Area,

$$
\begin{aligned}
\mathrm{A} & =\mathrm{L} * \mathrm{~B}=\mathrm{V} / \mathrm{H} \\
\Rightarrow \mathrm{~A} & =10.8 / 2.21
\end{aligned}
$$

$$
=4.89 \mathrm{~m}^{2}
$$

Assume, length to width ratio of 2

$$
\begin{aligned}
\Rightarrow \mathrm{L} & =2 \mathrm{~B} \\
\text { Now, Area, } \mathrm{A} & =\mathrm{L} * \mathrm{~B} \\
\Rightarrow>2 \mathrm{~B} * \mathrm{~B} & =4.89 \mathrm{~m}^{2} \\
\Rightarrow>\mathrm{B}^{2} & =4.89 / 2=2.44 \\
\Rightarrow \mathrm{~B} & =\sqrt{2.44}=1.56 \mathrm{~m}
\end{aligned}
$$

Therefore,

$$
\mathrm{L}=2 \mathrm{~B}
$$

$$
=2 * 1.56=3.12 \mathrm{~m}
$$

UG Water Tank Size

$$
\mathrm{L}=3.12 \mathrm{~m} \quad \mathrm{~B}=1.56 \mathrm{~m} \quad \mathrm{H}=2.36 \mathrm{~m}
$$

### 3.1.3 Over Head Tank (OHT)

In this part of the report the available size provided or set by the Architect is being checked. If architectural design is not available than the size is designed. Larger OHT size leads to higher live load. As it has been decided to pump twice everyday, therefore the size of OHT should include $50 \%$ daily demand, dead volume and volume for free board. This results in the size is about $60 \sim 70 \%$ of daily demand.
Daily water demand, $\mathrm{V}=5.4 \mathrm{~m}^{3} /$ day
It is decided that the users will pump $50 \%$ of the daily demand twice a day.
So, the required volume of OHT water tank

$$
\mathrm{V}=0.5 * 5.4=2.7 \mathrm{~m}^{3}
$$

Depth of OHT,

$$
\mathrm{H}=\mathrm{V}^{113}=(2.7)^{1 / 3}=1.39 \mathrm{~m}
$$

Free board, $\mathrm{FB}=0.15 \mathrm{~m}$
Total depth, $\mathrm{H}=\mathrm{H}+\mathrm{FB}=1.39+0.15=1.54 \mathrm{~m}$
Tank Area,

$$
\mathrm{A}=2.7 / 1.54=1.75 \mathrm{~m}^{2}
$$

Assume, $\mathrm{L}=2 \mathrm{~B}$

$$
\begin{aligned}
\mathrm{A} & =2 \mathrm{~B} * \mathrm{~B}=1.75 \mathrm{~m}^{2} \\
\Rightarrow \mathrm{~B}^{2} & =1.75 / 2=0.87 \\
\Rightarrow \mathrm{~B} & =\sqrt{0.87}=0.93 \mathrm{~m}
\end{aligned}
$$

Therefore,

$$
\mathrm{L}=2 \mathrm{~B}=2 * 0.93=1.86 \mathrm{~m}
$$

OHT water tank Size

$$
\mathrm{L}=1.86 \mathrm{~m} \quad \mathrm{~B}=0.93 \mathrm{~m} \quad \mathrm{H}=1.54 \mathrm{~m}
$$

Figure for the tank with pump and risers is shown in CAD \#12.

### 3.1.4 Pump Selection

Pump capacity should be such that, it matches the electricity type (volt) and should be able to pump the desired volume within 30 minutes or so.

Estimation of power required for the pump

$$
\begin{aligned}
& K W=\frac{Q(\mathrm{l} / \mathrm{s}) \times \operatorname{Head}(\mathrm{m})}{75 \times \text { Efficiency }} \times 0.746 \\
& \mathrm{Q}=\text { pump } 2.7 \mathrm{~m}^{3} / \mathrm{hr} . \\
&=\frac{2700 \mathrm{lts}}{1 * 60 * 60}=0.75
\end{aligned} \quad \begin{aligned}
\text { Here, } \quad \begin{aligned}
& \\
\text { Efficiency } & =50 \% \\
\text { and, Head } & =(15.42+0.75)=16.17 \mathrm{~m} \\
\mathrm{KW} & =\frac{0.75 * 16.17}{75 * 0.5} * 0.746=0.24
\end{aligned}
\end{aligned}
$$

### 3.1.5 Riser Size

Riser size is related to pump delivery pipe diameter. With the pumping rate, head and diameter of the pipe, head loss can be calculated. The water velocity should not exceed $1.5 \mathrm{~m} / \mathrm{s}$ (to minimize water hammer). Typical figure for risers along with the OHT and UG reservoir and pump is shown in CAD \#12.

### 3.2 Distribution Piping Design

CAD figures are provided showing riser, and distribution piping over the false ceiling with all the branches. BNBC provides the typical standard diameter for these pipes. For example all risers to the fixtures to be 13 mm dia ( 0.5 inch ) pipes, while pipes running over the false ceiling are 19 mm in diameter ( 0.75 inch). But, those standards should be checked for pressure drop study.

### 3.2.1 Pressure Check For Distribution Network.

The distribution pipeline diameters are checked for pressure. The objective of this calculation is to ensure minimum water pressure in the most difficult fixture. Most difficult fixture is generally located on the top floor with the highest pipe length from the source. The calculation table also helps to identify any inadequacy in the preliminary design and take corrective measures, i.e. adjust the diameter of the pipes which produce excessive head loss.

An isometric sketch showing the pipe layout on the roof and the risers were created to better understand this design and head-loss calculation. CAD \#5, CAD \#07 and CAD \#08 are used to get a total picture of that part of network which are used to deliver water to the most difficult fixture.

The Hazen Williams equation is used for head-loss calculations. The equation can be written in the following form:

$$
Q=3.7 \times 10^{-6} C \quad D^{2.63}\left(\frac{H}{L}\right)^{0.54}
$$

Where, $\mathrm{Q}=$ Flows, lps
C $=$ Roughness coefficient ( $100-140$ for rough to smooth pipes)
$\mathrm{D}=$ Diameter, mm
$\mathrm{H}=$ Head loss, m
$\mathrm{L}=$ Length of pipe, m
For a definite value of C , the equation can be written as:

$$
\frac{H}{L}=1.39 \times 10^{6} \frac{Q^{1.85}}{D^{4.87}} \quad---------[\text { for } \mathrm{C}=130]
$$

This above formula is used to calculate the head-loss or pressure check. WSFU were obtained from BNBC Table 1 in Appendix: BNBC Tables. Besides BNBC Table 2 is used to get the equivalent lengths for different valves and fittings. BNBC figure P1 is used to convert WSFU values to lpm values. For lower range of values a simple conversion factor is used as it is very difficult to obtain lpm values in that range.

Table 3.1: Analysis and design of distribution pipes based on head loss calculation Khitchen \& Toilet

| Pipe | Length <br> $\mathbf{L}$ <br> $(\mathbf{m})$ | Diameter <br> $\mathbf{D}$ <br> $(\mathbf{m m})$ | WSFU | (lpm) | (lps) | Unit loss <br> $\mathbf{H}_{\mathbf{f}} / \mathbf{L}$ <br> $(\mathbf{m} / \mathbf{m})$ | Loss <br> $\mathbf{H}_{\mathbf{f}}(\mathbf{m})$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 |
| A-Elbow | 1.52 | 38 | 52 | 93.6 | 1.56 | 0.07 | 0.10 |
| AB | 2.62 | 38 | 52 | 93.6 | 1.56 | 0.06 | 0.17 |
| B-Elbow | 1.52 | 38 | 52 | 93.6 | 1.56 | 0.07 | 0.10 |
| BC | 3.71 | 38 | 52 | 93.6 | 1.56 | 0.06 | 0.24 |
| C-Tee, St | 0.46 | 38 | 52 | 93.6 | 1.56 | 0.07 | 0.03 |
| CD | 4.78 | 38 | 52 | 93.6 | 1.56 | 0.06 | 0.31 |
| D-Tee, St | 0.46 | 38 | 52 | 93.6 | 1.56 | 0.07 | 0.03 |
| DE | 3.88 | 38 | 52 | 93.6 | 1.56 | 0.06 | 0.25 |
| E-Elbow | 1.52 | 38 | 52 | 93.6 | 1.56 | 0.07 | 0.10 |
| EF | 4.67 | 38 | 52 | 93.6 | 1.56 | 0.06 | 0.30 |
| F-Elbow | 1.52 | 38 | 52 | 93.6 | 1.56 | 0.07 | 0.10 |
| FG | 0.47 | 38 | 52 | 93.6 | 1.56 | 0.06 | 0.03 |


| Pipe | Length <br> $\mathbf{L}$ <br> $\mathbf{( m )}$ | Diameter <br> $\mathbf{D}$ <br> $(\mathbf{m m})$ | WSFU | (lpm) | (lps) | Unit loss <br> $\mathbf{H}_{\mathbf{f}} / \mathbf{L}$ <br> $(\mathbf{m} / \mathbf{m})$ | Loss <br> $\mathbf{H}_{\mathbf{f}}(\mathbf{m})$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| G-Elbow | 0.91 | 25 | 13 | 23.4 | 0.39 | 0.03 | 0.03 |
| GH | 2.44 | 25 | 13 | 23.4 | 0.39 | 0.04 | 0.09 |
| H-Elbow, St | 0.27 | 25 | 13 | 23.4 | 0.39 | 0.03 | 0.03 |
| I-Elbow | 0.76 | 19 | 4 | 7.2 | 0.12 | 0.01 | 0.01 |
| IJ | 0.64 | 19 | 4 | 7.2 | 0.12 | 0.02 | 0.03 |
| J- Elbow | 0.76 | 19 | 4 | 7.2 | 0.12 | 0.01 | 0.01 |
|  |  |  |  |  |  |  | 1.96 |

Total Head $\quad=5.03 \mathrm{~m}$
Head loss $\quad=1.96 \mathrm{~m}$
Available Head $=3.07 \mathrm{~m}=4.36 \mathrm{psi}$

| Pipe | Length <br> $\mathbf{L}$ <br> $\mathbf{( m )}$ | Daimeter <br> $\mathbf{D}$ <br> $(\mathbf{m m})$ | WSFU | (lpm) | (lps) | Unit loss <br> $\mathbf{H}_{\mathbf{f}} / \mathbf{L}$ | Loss <br> $\mathbf{H}_{\mathbf{r}}(\mathbf{m})$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 |
| A- Elbow | 1.56 | 38 | 52 | 93.6 | 1.56 | 0.7 | 0.10 |
| AB | 2.62 | 38 | 52 | 93.6 | 1.56 | 0.6 | 0.17 |
| B- Elbow | 1.52 | 38 | 52 | 93.6 | 1.56 | 0.7 | 0.10 |
| BC | 3.71 | 38 | 52 | 93.6 | 1.56 | 0.6 | 0.24 |
| C- Tee, St | 0.46 | 38 | 52 | 93.6 | 1.56 | 0.7 | 0.03 |
| CD | 4.78 | 38 | 52 | 93.6 | 1.56 | 0.6 | 0.31 |
| D- Tee, St | 0.46 | 38 | 52 | 93.6 | 1.56 | 0.7 | 0.03 |
| DE | 0.68 | 38 | 52 | 93.6 | 1.56 | 0.6 | 0.04 |
| E- Elbow | 1.52 | 38 | 52 | 93.6 | 1.56 | 0.7 | 0.10 |
| EF | 2.44 | 38 | 52 | 93.6 | 1.56 | 0.7 | 0.16 |
| F- Elbow | 0.91 | 25 | 13 | 23.4 | 0.39 | 0.03 | 0.03 |
| FG | 1.00 | 25 | 13 | 23.4 | 0.39 | 0.04 | 0.04 |
| G- Tee, St | 0.27 | 25 | 13 | 23.4 | 0.39 | 0.04 | 0.01 |
| GH | 1.16 | 25 | 13 | 23.4 | 0.39 | 0.03 | 0.04 |
| H- Elbow | 0.91 | 25 | 13 | 23.4 | 0.39 | 0.03 | 0.03 |
| GI | 1.11 | 25 | 13 | 23.4 | 0.39 | 0.04 | 0.04 |
| I- Elbow | 0.91 | 25 | 13 | 23.4 | 0.39 | 0.03 | 0.03 |
|  |  |  |  |  |  |  | 1.5 |

Total Head $=5.03 \mathrm{~m}$
Head loss $\quad=1.5 \mathrm{~m}$
Available Head $=3.53 \mathrm{~m}=5.02 \mathrm{psi}$

### 3.2.2 Typical Figure For Connections

Typical standard for valves, joints, clumps etc. are also provided in the CAD.

## CHAPTER 4 DESIGN OF SEWERAGE SYSTEM

### 4.1 General

The detailed calculations for the design of different components are presented in this chapter. The sewerage plumbing pipes for each floors are generally located below the floor slab and the vertical stacks run through the designated voids to the building drains.

A waste water drainage system design requires the following components to be considered:

- Determine the wastewater load in terms of fixture unit from each bathroom or other units which produces wastewater.
- Selection / Check adequacy of the size (diameter) of the branch pipes according to BNBC.
- Selection / Check adequacy of the size (diameter) of the vertical stack pipes according to BNBC.
- Determining the grade and diameter of the building drain.
- Design the septic tank.


### 4.2 Wastewater Load

There are six toilet unit locations and three kitchen location in the upper two floors while the bottom 2 floors have some less number of toilets due to different space arrangements. All these kitchens and toilets are using a total of six (6) voids to run down the vertical stacks. Waste water loads are calculated based on the Fixture Unit (FU) system. In this system a load value is assigned for each type of wastewater fixture. The relevant table from BNBC [Table 8.7.14] is provided in Appendix: BNBC Tables. FU values are picked up from that table to determine the total FU load coming from each toilet to each type of vertical stacks. The fixture unit loads to different stacks from these toilets are listed in the following table:

Table 4.1: Listing of fixutue unit loads from each toilet and kitchen (2 $2^{\text {nd }} \& 3^{r d}$ floor)

| SI. \# | Description | Fixture Unit (FU) for Waste Stack (WS) | Fixture Unit <br> (FU) for Soil <br> Stack (SS) | Void <br> Area |
| :---: | :---: | :---: | :---: | :---: |
| 1 | Toilet 1 | Floor Trap (3) + Shower (1) + Wash Basin (1) + <br> Laundry (1) | Water Closet (6) | 1 |
| 2 | Toilet 2 | Floor Trap (3) + Shower (1) + Wash Basin (1) + <br> Laundry (1) | Water Closet (6) | 2 |
| 3 | Toilet 3 | Floor Trap (3) + Shower (1) + Wash Basin (1) + <br> Laundry (1) | Water Closet (6) | 3 |
| 4 | Toilet 4 | Floor Trap (3) + Shower (1) + Wash Basin (1) + <br> Laundry () | Water Closet (6) | 4 |
| 5 | Kitchen 1 | Sink (1) | - | 2 |
| 6 | Kitchen 2 | Sink (1) | - | 5 |

These pipe generally runs below the floor and above the false ceiling of the floor below. The floor plan is different in $1^{\text {st }}$ floor, the fixture loads from this floor is listed below:

Table 4.2: Listing of fixutue unit loads from each toilet and kitchen (1st floor)

| Sl. \# | Description | Fixture Unit (FU) for Waste Stack (WS) | Fixture Unit <br> (FU) for Soil <br> Stack (SS) | Void <br> Area |
| :---: | :---: | :---: | :---: | :---: |
| 1 | Toilet 1 | Floor Trap (3) + Shower (2) + Wash Basin (1) + <br> Laundry (2) | Water Closet (6) | 1 |
| 2 | Toilet 2 | Floor Trap (3) + Shower (1) + Wash Basin (1) + <br> Laundry (2) | Water Closet (6) | 2 |
| 3 | Toilet 3 | Floor Trap (3) + Shower (1) + Wash Basin (1) + <br> Laundry (2) | Water Closet (6) | 3 |
| 4 | Toilet 4 | Floor Trap (3) + Shower (2) + Wash Basin (1) + <br> Laundry (2) | Water Closet (6) | 4 |
| 5 | Kitchen 2 | Sink (1) | - | 1 |

Total fixture load on each stack is listed below:

Table 4.3: List of Wastewater fixture loads on different stacks (Ground floor $\sim 3^{\text {rd }}$ floor)

| Void area designation | Fixture unit load on Waste Stack (WS) |  |  |  |  | Fixture unit load on Soil Stack (SS) |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | GF | 1F | 2F | 3F | Total | GF | 1F | 2F | 3F | Total |
| 1 | 0 | 8 | 6 | 6 | 20 | 0 | 6 | 6 | 6 | 18 |
| 2 | 0 | 7 | 6 | 6 | 19 | 0 | 6 | 6 | 6 | 18 |
| 3 | 0 | 7 | 6 | 6 | 19 | 0 | 6 | 6 | 6 | 18 |
| 4 | 0 | 8 | 6 | 6 | 20 | 0 | 6 | 6 | 6 | 18 |
| 5 | 0 | 1 | 1 | 1 | 03 |  |  |  |  |  |

CAD \#11 includes the section detail of fixtures.
Table 4.4 : List of design of building drainage

| Pipe | Length <br> $(\mathrm{cm})$ | FU | Diameter <br> $(\mathrm{mm})$ | Slope | Difference <br> in elevation <br> $(\mathrm{mm})$ | Check -pit | Elevation <br> $(\mathrm{mm})$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| AB | 3.276 | 270 | 150 | $\frac{1}{100}$ | 242.77 | A | 774.49 |
| BC | 4.572 | 390 | 150 | $\frac{1}{100}$ | 209.70 | B | 531.72 |
| CD | 5.410 | 390 | 150 | $\frac{1}{100}$ | 163.98 | C | 322.02 |
| DE | 6.172 | 510 | 150 | $\frac{1}{100}$ | 109.88 | D | 158.04 |
| EF | 4.815 | 510 | 150 | $\frac{1}{100}$ | 48.16 | E | 48.16 |
|  |  |  |  | Total $=$ | 774.49 | F | 0 |

### 4.3 Selection Of Branch And Stack Pipe Size

All branch pipes are selected to be 100 mm in diameter. This uniform size selection will eliminate the need of reducers and will reduce the hassle of maintaining different sized pipe stocks for any repair work. According to Table 8.7.16 from BNBC (Appendix: BNBC Tables) this 100 mm branch pipe can take up to 160 FU load and hence this selection is more than enough. That table also indicate that the vertical stacks of this size can take upto 500 FU load and hence this design or selection is enough. CAD\#10 includes a sketch of stack with velocity reducer arrangement. CAD \#13 includes typical wastewater and rain pipe joint details.

### 4.4 Design Of Building Drain

Total load on building drains for SS and WS are 138 and 183 FU accordingly. Therefore, according to BNBC Table 8.7.17 (Appendix: BNBC Tables), Ø150 mm pipe is selected with a slope of $1 / 100$. This setup has a capacity of 700 FU load.

Layout of the building drain with, Septic tank, building sewer etc are shown in CAD \#05. A layout of the UG reservoir for water supply is shown in CAD \#5. These two elements are separated by sufficient distance to prevent any chance of cross contamination. CAD\#12 and CAD\#18 shows the detailed of stack connections and detailed sections of typical inspection pits for building drains.

### 4.5 Design Of Septic Tank.

Septic tank has been designed according to the process described in the text book. All the CAD for septic tank and soak well arrangements are provided in CAD \#9 and CAD \#10.

## ChAPTER 5 <br> Design of Storm Water Drainage

### 5.1 General

There is no special arrangements to store and manage storm water for this building. Arrangements were made to discharge rain water easily from roof through several rain stacks to the building drain.

### 5.2 Storm Water Management System

There are seven zones of vertical pipe groups in this building. Five (5) of these zones include a rain drainage pipe / stack. CAD \#08 shows the drainage plan for the storm/ rain water from roof with the slope directions and rain pipe locations.
CAD \#14: AC wastewater system

## REFERENCES

1. McGraw hill book company, Water quality and treatment, 1990
2. Ahmed, F.and smith, P.G, Design and performance of a community type iron removal plant for pump tubewell, 1987
3. [ From BNBC 2020 Table 5.9.1.2.c], BNBC [ Table 8.7.14], BNBC Table Table 8.7.17 (Appendix : BNBC Table),(BNBC Table P1),(BNBC Table P2),Table 8.7.16 [BNBC]:, Table 8.7.17[BNBC].

## APPENDICES

Appendix: BNBC Tables

Fig. P 1 Water Supply Demand for Various Loads in Water Supply Fixture Units (wsfu)

Table 1: Water Supply Fixture Unit (wsfu) Values for Various Plumbing Fixtures (BNBC Table P1)

| Fixtures of Group | Supply Control | wsfu |  |  |
| :---: | :---: | :---: | :---: | :---: |
|  |  | Cold | Hot | Total |
|  |  |  |  |  |
| Bath Group | Flush tank | 4.5 | 3 | 6 |
| Bath Group | Flush valve | 6 | 3 | 8 |
| Bathtub | Faucet | 1.5 | 1.5 | 2 |
| Bidet | Faucet | 1.5 | 1.5 | 2 |
| Combination | Faucet | 2 | 2 | 3 |
| Kitchen sink | Faucet | 1.5 | 1.5 | 2 |
| Laundry tray | Faucet | 2 | 2 | 3 |
| Laundry | Faucet | 1.5 | 1.5 | 2 |
| Pedestal urinal | Flush valve | 10 | - | 10 |
| Restaurant sink | Faucet | 3 | 3 | 4 |
| Service sink | Faucet | 1.5 | 1.5 | 2 |
| Shower head | Mixing Valve | 3 | 3 | 4 |
| Stall or wall urinal | Flush tank | 3 | - | 3 |
| Stall or wall urinal | Flush valve | 5 | - | 5 |
| Water closet | Flush tank | 5 | - | 5 |
| Water closet | Flush valve | 10 | - | 10 |
| Fixture with both cold and hot water supplies, the weight for maximum separate demands |  |  |  |  |
| * may be considered 75\% of total wsfu. |  |  |  |  |

Table 2: Equivalent Length of Pipe for Friction Loss in Valves and Fittings (BNBC Table P2)

| Valves or <br> Fittings | Equivalent Length (m) of Pipes Against Diameter (mm) of Fittings |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 10 | 13 | 19 | 25 | 32 | 38 | 50 | 63 | 75 | 88 | 100 | 125 | 150 |
| Angle Valve | 1.22 | 2.44 | 3.66 | 4.57 | 5.49 | 6.71 | 8.53 | 10.36 | 12.19 | 15.24 | 16.76 | 21.34 | 24.38 |
| Gate Valve | 0.06 | 0.12 | 0.15 | 0.18 | 0.24 | 0.30 | 0.40 | 0.49 | 0.61 | 0.73 | 0.82 | 1.01 | 1.22 |
| Glove Valve | 2.44 | 4.57 | 6.10 | 7.62 | 10.67 | 13.72 | 16.76 | 19.81 | 24.38 | 30.48 | 38.10 | 42.67 | 50.29 |
| $90^{\circ}$ Standard <br> Elbow | 0.30 | 0.61 | 0.76 | 0.91 | 1.22 | 1.52 | 2.13 | 2.44 | 3.05 | 3.66 | 4.26 | 5.18 | 6.10 |
| $45^{\circ}$ Standard <br> Elbow | 0.18 | 0.37 | 0.46 | 0.55 | 0.73 | 0.91 | 1.22 | 1.52 | 1.83 | 2.13 | 2.44 | 3.05 | 3.66 |
| $90^{\circ}$ Side Tee <br> Coupling <br> Straight Run of <br> Tee | 0.46 | 0.91 | 1.22 | 1.52 | 1.83 | 2.13 | 3.05 | 3.66 | 4.57 | 5.49 | 6.40 | 7.62 | 9.14 |

Table 8.7.14
Fixture Units for Different Sanitary Appliances or Groups

| Type of Fixture | Fixture Unit Value as Load Factors |
| :---: | :---: |
| One bathroom group consisting of water closet, wash basin and bath tub or shower stall : |  |
| a) Flush Tank water closet | 6 |
| b) Flush-valve water closet | 8 |
| Bathtub* | 3 |
| Bidet | 3 |
| Combination sink and tray (drain board) | 3 |
| Drinking fountain | 0.5 |
| Floor trapst | 1 |
| Kitchen sink, domestic | 2 |
| Wash basin, ordinary $\ddagger$ | 1 |
| Wash basin, surgeon's | 2 |
| Shower stall, domestic | 2 |
| Shower (group) per head | 3 |
| Urinal, wall lip | 4 |
| Urinal, stall | 4 |
| Water closet, tank operated | 4 |
| Water closet, valve operated | 8 |
| * A shower head over a bath tub does not increase the fixture unit value. <br> + Size of floor trap shall be determined by the area of surface water to be drained. <br> $\ddagger$ Wash basin with 32 mm and 40 mm trap have the same load value. |  |

Table 8.7.16 [BNBC]:
Maximum number of Fixture Units that can be Connected to Branches and Stacks

| $\begin{aligned} & \text { Diameter of } \\ & \text { Pipe } \\ & (\mathrm{mm}) \end{aligned}$ | Maximum Number of Fixture Units that can be Connected |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
|  | Any Horizontal Fixture Branch | One Stack of 3 <br> Storeys in Height or 3 Intervals | More than 3 Storeys in Height |  |
|  |  |  | Total for stack | Total at One Storey or Branch Interval |
| 30 | 1 | 2 | 2 | 1 |
| 40 | 3 | 4 | 8 | 2 |
| 50 | 6 | 10 | 24 | 6 |
| 65 | 12 | 20 | 42 | 9 |
| 75 | 20 | 30 | 60 | 16 |
| 100 | 160 | 240 | 500 | 90 |
| 125 | 360 | 540 | 1100 | 200 |
| 150 | 620 | 960 | 1900 | 350 |
| 200 | 1400 | 2200 | 3600 | 600 |
| 250 | 2500 | 3800 | 5600 | 1000 |
| 300 | 3900 | 6000 | 8400 | 1500 |
| 375 | 7000 | b | b | b |
| a Does not include branches of the building sewer. <br> b Sizing load based on design criteria. |  |  |  |  |

Table 8.7.17 [BNBC]: Maximum number of Fixture Units that can be Connected to Building Drains and Sewers

| Diameter of <br> Pipe <br> $(\mathrm{mm})$ | Maximum Number of Fixture Units that can be Connected to any <br> Portion* of the Building Drain or the Building Sewer for Various <br> Slopes |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
|  | $1 / 200$ | $1 / 100$ | $1 / 50$ |  |
| 100 | - | 180 | 216 |  |
| 150 | - | 700 | 840 |  |
| 200 | 1400 | 1600 | 1900 |  |
| 250 | 2500 | 2900 | 3500 |  |
| 300 | 2900 | 4600 | 5600 |  |
| 375 | 7000 | 8300 | 10000 |  |

Appendix: CAD Files












LEGEND: (For Sewerage Disposal)
SS = 4"Ø UPVC SOIL STACK
WS $=4$ " $\emptyset$ UPVC WASTE STACK
RS= $=4 " \emptyset$ uPVC RAIN STACK.
RSI $=11 / 4 \emptyset$ uPVC RAIN STACK./AC WASTE WATER
$\mathrm{VS}=2$ " $\varnothing$ uPVC VENT STACK.
$\mathrm{FT}=\mathrm{FLOOR}$ TRAP.
$\mathrm{P}=11 / 2$ "Ø uPVC CONDUIT CONCEALED IN FLOOR
D = DEPTH OF INSPECTION CHAMBER FOR SOIL from FGL
$\mathrm{d}=$ DEPTH OF INSPECTION CHAMBER FOR
RAIN \& WASTE WATER from FGL
= INSPECTION CHAMBER FOR SOI
= INSPECTION CHAMBER FOR WASTE WATER
O = 24"Ø CI. MANHOLE COVER (HEAVY TYPE)


TOR Y Jolnt detall


PLAN OF ISPECTION PII


INSTALLATION OF WATER SUPPLY PUMP \& PIPE WOKK




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